

SECTION 009101

ADDENDUM 001

DATE: April 13, 2015

RE: CREMATORIUM ADDITION
For Animal Village, City of Little Rock, Arkansas

CoLR Bid# 15128
AE Project No.: 2014-205-10

FROM: CROMWELL ARCHITECT ENGINEERS, INC.
101 S. Spring Street
Little Rock, Arkansas 72201

TO: BIDDERS OF RECORD

This Addendum forms a part of the Contract Documents and modifies the original Bidding Documents dated 30-Jan-2015, as noted below. Acknowledge receipt of this Addendum in the space provided on the Bid Form. Failure to do so may subject Bidder to disqualification. Each item in Contract Documents complements each of the other Contract Documents. No sheet, section, or document is to be followed without referring to all sheets, sections, and parts of the Contract Documents.

CHANGES TO PROCUREMENT AND CONTRACTING REQUIREMENTS:

- 1: Insert new Section 00 31 00 - Available Project Information issued with this Addendum.
 - a. Insert the Geotechnical Report issued with this Addendum
- 2: Insert (this) Section 00 91 01 - Addendum 001 issued with this Addendum.

CHANGES TO SPECIFICATIONS:

- 3: Replace Section 00 11 00 - Notice To Bidders with revised Section issued with this Addendum.
- 4: Replace Section 32 31 13 - Chain Link Fences with revised Section issued with this Addendum.
- 5: Insert the attached revised and/or new Drawings issued with this Addendum:

Sheet	Title/Description	Issue Date	Revision No.
AD101	Demolition Floor Plan	04-13-2015	1

END OF SECTION

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SECTION 00 11 00
NOTICE TO BIDDERS

RE: ANIMAL VILLAGE CREMATORIUM ADDITION FOR CITY OF LITTLE ROCK

FROM:

The City of Little Rock, Arkansas (hereinafter referred to as Owner).

And the Design Firm of Cromwell Architects Engineers, 101 South Spring Street; Little Rock, Arkansas 72201; telephone: 501-372-2900 (hereinafter referred to as Architect Engineer).

1. Contact: Mr. Mark Littrell, AIA, <mlittrell@cromwell.com>.

TO: INTERESTED BIDDERS

- A. Time and Place: Sealed bids for the designated Work will be received by until 3:00 PM official local time at the place receiving bids, on April 21, 2015 and publicly opened at the offices of City of Little Rock Purchasing, 500 W Markham St, Suite 301, Little Rock, AR 72201 Attention: Abdoul Kabaou.
- B. Description: A Single Lump Sum Proposal is requested on a Contract for the Construction of Animal Village Quarantine Pens for the City of Little Rock in accordance with the Drawings and Specifications.
- C. Examination and Procurement Of Documents: Bidding and Contract Documents may be examined and/or obtained at the office of Cromwell Architects Engineers, 101 S. Spring, Little Rock, Arkansas 72201-2490, Telephone (501) 400-1006, Printshop@cromwell.com.
- D. Bid Security: A 5% Bid Security is required with Bid.
- E. Bid Requirements: The Owner reserves the right to reject bids and waive formalities. Bidder must have a current general or specialty license from the Arkansas Contractor's Licensing Board. The bid proposal and work of the proposed contract shall be in accordance with all applicable federal, state, county, and local laws, ordinances, and regulations.
- F. Mandatory Pre-Bid Conference:
 - 1. Time and Date: April 7, 2015 at 9:00 AM.
 - 2. Location: Animal Village located at 4500 Kramer Street, Little Rock, Arkansas.
- G. The City of Little Rock encourages participation of small, minority, and woman own business enterprises in the procurement of goods, services, professional services, and construction, either as a general contractor or sub-contractor. It is further requested that whenever possible, majority contractors who require sub-contractors, seek qualified small, minority, and woman businesses to partner with them.
- H. Refer to other bidding requirements described in Section 00 20 00 - Instructions to Bidders.

END OF SECTION 00 11 00

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SECTION 00 31 00
AVAILABLE PROJECT INFORMATION

PART 1 GENERAL

1.01 EXISTING CONDITIONS

- A. Certain information relating to existing surface and subsurface conditions and structures is available to bidders but will not be part of the Contract Documents, as follows:
- B. Geotechnical Report: Entitled Results of Geotechnical Investigation Animal Village Crematorium Addition, dated December 26, 2014.
 - 1. Prepared by Grubbs, Hoskyn, Barton & Wyatt, Inc., Little Rock, Arkansas.
 - 2. For Contractor's convenience a copy is included following end of this section.
 - 3. This report identifies properties of below grade conditions and offers recommendations for the design of foundations, prepared primarily for the use of the Architect Engineer.
 - 4. The recommendations described shall not be construed as a requirement of this Contract, unless specifically referenced in the Contract Documents.
 - 5. This report, by its nature, cannot reveal all conditions that exist on the site. Should subsurface conditions be found to vary substantially from this report, changes in the design and construction of foundations will be made, with resulting credits or expenditures to the Contract Sum accruing to Owner.

PART 2 PRODUCTS (NOT USED)

PART 3 EXECUTION (NOT USED)

END OF SECTION

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P.O. Box 30970
Little Rock, Arkansas 72260-0970
#1 Trigon Place 72209
(501) 455-2536
FAX (501) 455-4137

December 26, 2014
Job No. 14-182

Cromwell Architects Engineers
101 South Spring Street
Little Rock, Arkansas 72201

Attn: Mr. Mark Littrell, AIA

**REF: RESULTS of GEOTECHNICAL INVESTIGATION
ANIMAL VILLAGE CREMATORIUM ADDITION
LITTLE ROCK, ARKANSAS
CROMWELL PROJECT No. 2014-205-10**

INTRODUCTION

Presented herein are the results of the geotechnical investigation performed for the new crematorium planned at Animal Village in Little Rock, Arkansas. This study was authorized by Cromwell Architects Engineers on November 2, 2014. This geotechnical investigation has been performed in general accordance with our proposal of November 18, 2014 (GHBW Proposal No. 14-232). Preliminary results and recommendations were provided on December 19, 2014.

We understand that the crematorium will be about 27 ft by 33 ft in plan dimension. The facility will include an incinerator. Specific information on the facility has not been provided. However, foundation loads are expected to be light. Site grading information is not currently available, but site grading is expected to be minor.

The purposes of this study were to explore subsurface conditions at the crematorium addition location and to develop recommendations to guide design and construction of foundations and construction criteria. The results of the field and laboratory studies are discussed in the following report sections. Subsequent report sections provide recommendations for design and construction.

SUBSURFACE EXPLORATION

Subsurface conditions at the Animal Village crematorium site were explored by drilling one (1) sample boring (Boring 1) to auger refusal at 17.5-ft depth. The scope of the field studies and the boring location were specified by the Architect. The site vicinity is shown on Plate 1. The approximate boring location is shown on the Plan of Borings, Plate 2. The boring log, presenting descriptions of the subsurface strata encountered and results of field and laboratory tests, is included as Plate 3. A key to the terms and symbols used on the logs is presented as Plate 4.

Subsurface conditions at the Animal Village addition location were also explored by drilling one (1) sample boring at the plan quarantine addition location, west of the crematorium (see Boring A). In addition, subsurface conditions were previously explored during the geotechnical investigation performed in 2005 (GHBW Project No. 05-282). The log of Boring A and selected

borings from the 2005 study are provided in Appendix A. The approximate boring locations are shown on Plate 2. The results of the boring drilled for the quarantine pens and the borings drilled for the 2005 study were utilized where appropriate.

Boring 1 was drilled with a truck-mounted SIMCO 2400 rotary-drilling rig using dry-auger drilling techniques. Samples were obtained using a 2-in.-diameter split-barrel sampler driven into the strata by blows of a 140-lb safety hammer with 30-in. drop in accordance with Standard Penetration Test (SPT) procedures. The number of blows required to drive the standard split-barrel sampler the final 12 in. of an 18-in. total drive, or a portion thereof, is defined as the Standard Penetration Number (N). Recorded N-values are shown on the boring logs in the "Blows Per Ft" column. The boring was terminated at auger refusal.

All samples were removed from sampling tools in the field, examined and visually classified. Samples were then placed in appropriate containers to prevent moisture loss and/or change in condition during transfer to our laboratory for further examination and testing.

The boring was advanced using dry-auger drilling procedures to facilitate groundwater observations. Observations regarding groundwater are noted in the lower-right portion of each log and are discussed in subsequent sections of this report. The borehole was backfilled after obtaining final groundwater readings.

LABORATORY TESTING

To evaluate pertinent physical and engineering characteristics of the foundation and subgrade soils, laboratory tests consisting of natural water content determinations and classification tests were performed on selected representative soil samples. Seven (7) natural water content determinations were performed to develop a water content profile for the boring. The results of these tests are plotted on the logs as solid circles, in accordance with the scale and symbols shown in the legend located in the upper-right corner.

To verify field classification and to evaluate soil plasticity, two (2) liquid and plastic (Atterberg) limit determinations and two (2) sieve analyses were performed on selected representative samples. The Atterberg limits are plotted on the logs as pluses inter-connected with a dashed line using the water content scale. The percent of soil passing the No. 200 Sieve is noted in the "Minus No. 200" column on the log forms.

GENERAL SITE and SUBSURFACE CONDITIONS

Site Conditions

The project site is located at the existing Animal Village facility at 4500 Kramer Street in Little Rock, Arkansas. The crematorium addition will be located on the northwest corner of the existing building. The crematorium location is an area where site grades were apparently raised for construction of the main building. The terrain at the addition location is flat, though several feet higher than the terrain to the north and west. The area is currently an animal pen and the ground surface is largely exposed earth. Surface drainage is considered fair to good.

Seismic Conditions

Based on Arkansas State criteria, the Pulaski County site is located in Seismic Zone 1, designated as the "Area of low anticipated seismic damage. In light of the soil conditions revealed by the borings performed for this study and for the prior studies, a Seismic Site Class C (very stiff

soil and soft rock profile) is considered fitting with respect to International Building Code (IBC) criteria (IBC 2012).

Subsurface Conditions

The site is underlain by the lower Pennsylvanian Period Jackfork Sandstone formation. The Jackfork is comprised of thin- to massive-bedded fine- to coarse-grained quartzitic sandstone with subordinate silty sandstone and shale units. The sandstone sometimes occurs as discontinuous masses in the shale. The thickness of the formation varies from 3500 to 6000 feet.

On-site fill comprised of loose to medium dense gray crushed syenite fines extends to about 6-ft depth. The upper zones of the on-site fill contain organics. The syenite fines on-site fill are poorly graded and poorly compacted with low relative density to about 4-ft depth, but increasing relative density below 4 feet. The upper zones of the on-site fill are weak with high compressibility. Below the syenite fines on-site variable fill comprised of stiff tan sandy, silty clay and medium dense brown, tan and gray clayey fine to coarse sand with shale and quartz fragments extends to about 12-ft depth. The deeper zones of the on-site fill, below about 4-ft depth, are relatively compact with moderate shear strength/medium relative density and low compressibility. The content, depth, and compaction of the on-site fill are likely to vary across the site.

The on-site fill is underlain by firm brown to tan and light gray fine sandy clay. The sandy clay has low shear strength and moderate compressibility.

The results of the borings drilled in 2005 indicate that shale is present below the variable on-site fill and the overburden soils. The shale represents bedrock of the Jackfork Sandstone formation.

Groundwater was not encountered within the exploration depth of the boring drilled in November 2014. However, perched water may be present in the variable on-site fill and overburden soils as well as the fractured zones of the shale and sandstone bedrock. Groundwater levels will be influenced by seasonal precipitation and surface infiltration and should be expected to vary with seasonal site conditions. Though not encountered in the borings, seasonal springs or seeps could be present as surface water flows from the higher terrain around the existing building.

ANALYSES and RECOMMENDATIONS

Foundations

Based on the results of the boring drilled for this study and the prior borings drilled at the Animal Village site and the anticipated light foundation loads of the crematorium addition, shallow footings are considered suitable for support of light structural loads. In the event that foundation loads exceed about 50 kips or wall loads exceed about 5 kips per linear ft, consideration should be given to utilizing drilled pier or rammed aggregate pier (Geopier[®] elements) foundation systems. Recommendations for footings are discussed in the following report sections. Recommendations for foundation alternatives for support of moderate structural loads can be provided upon request.

Footing Foundations

Light foundation loads may be supported on footings bearing in the compact on-site fill or compacted select fill at a minimum depth of 1.5 ft below lowest adjacent grade. The depth of footings adjacent to the existing building should match the existing footing bottom elevations to

the extent possible. Care must be taken not to undermine existing footings or floor slabs with the new foundation excavations.

Footings founded as recommended may be designed based on a maximum net allowable soil bearing pressure of 1500 lbs per sq ft for continuous and individual footings. The recommended allowable bearing pressure is based on a minimum factor of safety of 2.5 with respect to anticipated shear strength of compact on-site fill and compacted select fill. Post-construction settlement of lightly-loaded footings founded as recommended should be less than 1 inch.

Some localized undercut is likely to be required to develop suitable bearing. The results of Boring 1 indicate that the syenite fines on-site fill is loose to about 4-ft depth. Consequently, localized undercuts on the order of 4 ft, more or less, should be anticipated for foundation construction. Footings undercuts must extend to suitable bearing strata as field verified by the Geotechnical Engineer. The on-site syenite fines, free of organics or debris, may be utilized for select fill after undercutting, processing, and re-compaction. Alternatively, footing undercuts may be backfilled with select fill, flowable fill (minimum compressive strength 300 psi), or lean concrete. Undercuts should extend at least 5 ft outside the building limits to the extent possible. Footings supported on undercut backfill as recommended may be sized for the previously recommended bearing pressures

Footing undercuts backfilled with select fill should have a minimum width determined by a 1-horizontal to 2-vertical (1H:2V) projection from the edge of footings to the undercut bottom. Undercuts may be excavated neat to plan footing dimensions where backfilled with flowable fill or concrete. Where site conditions warrant mass undercut, footings may be founded in the compacted undercut backfill. Mass undercuts should extend at least 8 ft outside the building limits to the extent possible.

Uplift resistance of footings will be provided by the weight of the structure and the foundation units. Resistance to lateral forces will be developed by the passive resistance of the foundation soils and sliding resistance at the footing bottom. The passive resistance of the soil within the upper 1.5 ft should be neglected. Below 1.5-ft depth, an ultimate passive resistance value of 120 lbs per sq ft increasing at 120 lbs per sq ft per ft depth may be assumed for the compact on-site fill and compacted select fill. Resistance to sliding may also be evaluated using an ultimate friction value ($\tan \delta$) of 0.30 for concrete on suitable bearing strata. Where foundation undercuts are backfilled with flowable fill or concrete, these should be rough finished to enhance friction resistance. Alternatively, short dowels may be utilized to mobilize shear resistance between foundation bottoms and cementitious backfill. An appropriate factor of safety must be included in analysis of sliding.

Continuous footings should have a minimum width of 18 in. and individual footings a minimum dimension of 24 inches. A minimum footing depth of 1.5 ft below lowest adjacent grade is recommended for foundation embedment. All footing excavations and undercuts should be observed by the Geotechnical Engineer to verify suitable bearing and adequate undercut.

Floor Slab

Slab-on-grade construction is considered appropriate for the addition floor slab. Fill placed to achieve design floor grade should comply with the recommendations in the Site Grading section of this report. In addition, we recommend the slab be supported on a 4- to 6-in.-thick clean crushed

stone or gravel layer placed on a properly prepared subgrade. Impervious sheeting should be placed between the slab and the granular course to act as a vapor retarder.

Site Grading

Subgrade preparation in the addition area should begin with stripping all soft or organic-containing soils. Based on the observed depth of the organic zone, a stripping depth on the order of 12 in. is expected, though it could be deeper in heavily-organic areas.

All subgrade should be evaluated for the location and extent of soft soil zones by proof-rolling with loaded rubber-tired equipment or otherwise evaluated by the Geotechnical Engineer. Areas identified to be unstable, weak or that are otherwise unsuitable should be undercut and processed and re-compacted or replaced, whichever is appropriate. Localized undercuts on the order of 4 to 5 ft below existing grade, more or less, will be required for footings. In addition, localized undercuts could be required to develop a stable subgrade for floor slabs.

The on-site syenite fines, free of organics and debris, are suitable for use as fill and backfill. However, processing, segregation of organic-contaminated zones, and adjustment of the water content will be required. Imported borrow for fill or backfill should consist of approved select clayey sand (SC), sandy clay (CL), or clayey gravel (GC) having a liquid limit less than 40, or an approved alternate. Locally available syenite fines, i.e., "Donna-fill" or "Granufill Type 2 or Type 3" are also suitable borrow. Granular fill materials must be protected from erosion and contained. Donna-fill and Granufill Type 2 are particularly susceptible to erosion. All utility trenches that extend through areas of the on-site syenite fines, Donna-fill, or Granufill Type 2 should be bedded in crushed stone aggregate base and trenches completely backfilled with aggregate base. All fill and backfill should be free of organic materials and durable rock fragments in excess of about 3-in. dimension. All fill and backfill should be approved by the Geotechnical Engineer.

Fill, backfill, and recompacted soils should be compacted to a minimum of 95 percent of the Modified Proctor (ASTM D-1557) maximum dry density within a water content range of 2 percent below to 3 percent above the optimum value. Fill and backfill should be placed in nominal 6- to 8-in.-thick loose lifts. Each lift of backfill and fill should be tested and approved prior to placing subsequent lifts.

CONSTRUCTION CONSIDERATIONS

Positive surface and subsurface drainage should be established at the start of construction, maintained during the work, and incorporated into final design to prevent surface water ponding and subsequent saturation of subgrade and foundation soils. Shallow groundwater is not expected to impact the work. However, localized seeps or perched groundwater may be present or encountered during the work. Where seeps are encountered, seepage should be directed to positive discharge at daylight or into storm drainage lines via French drains or blanket drains.

Undercut of some footing locations is expected to be required. Care should be taken that the existing building foundations, floor slabs, and existing utilities are not undermined by undercuts or other excavations for the addition.

All foundation excavations and undercuts should be observed by the Geotechnical Engineer to verify suitable bearing and adequate undercut. Concrete should be placed in footing excavations expeditiously following final clean up and approval to limit changes in foundation conditions. Footing excavations should be clean and dry at the time of concrete placement. Where

footing excavations will be left open for extended periods, the bearing stratum should be protected with a thin layer of seal concrete.

CLOSING

The Architect or a designated representative should monitor site grading, subgrade preparation, and foundation construction. Subsurface conditions significantly at variance with those encountered in the borings should be brought to the attention of the Geotechnical Engineer and work delayed pending evaluation and/or preparation of additional recommendations, if warranted.

The following illustrations are attached and complete this report.

Plate 1	Site Vicinity
Plate 2	Plan of Borings
Plate 3	Boring Log
Plate 4	Key to Terms and Symbols
Appendix A	Selected Boring Logs from Other Studies

* * * *

We appreciate the opportunity to be of service to you during this phase of the project. Should you have any questions regarding this report, or if we may be of additional assistance during final design or construction, please call on us.

Sincerely,

**GRUBBS, HOSKYN,
BARTON & WYATT, INC.**

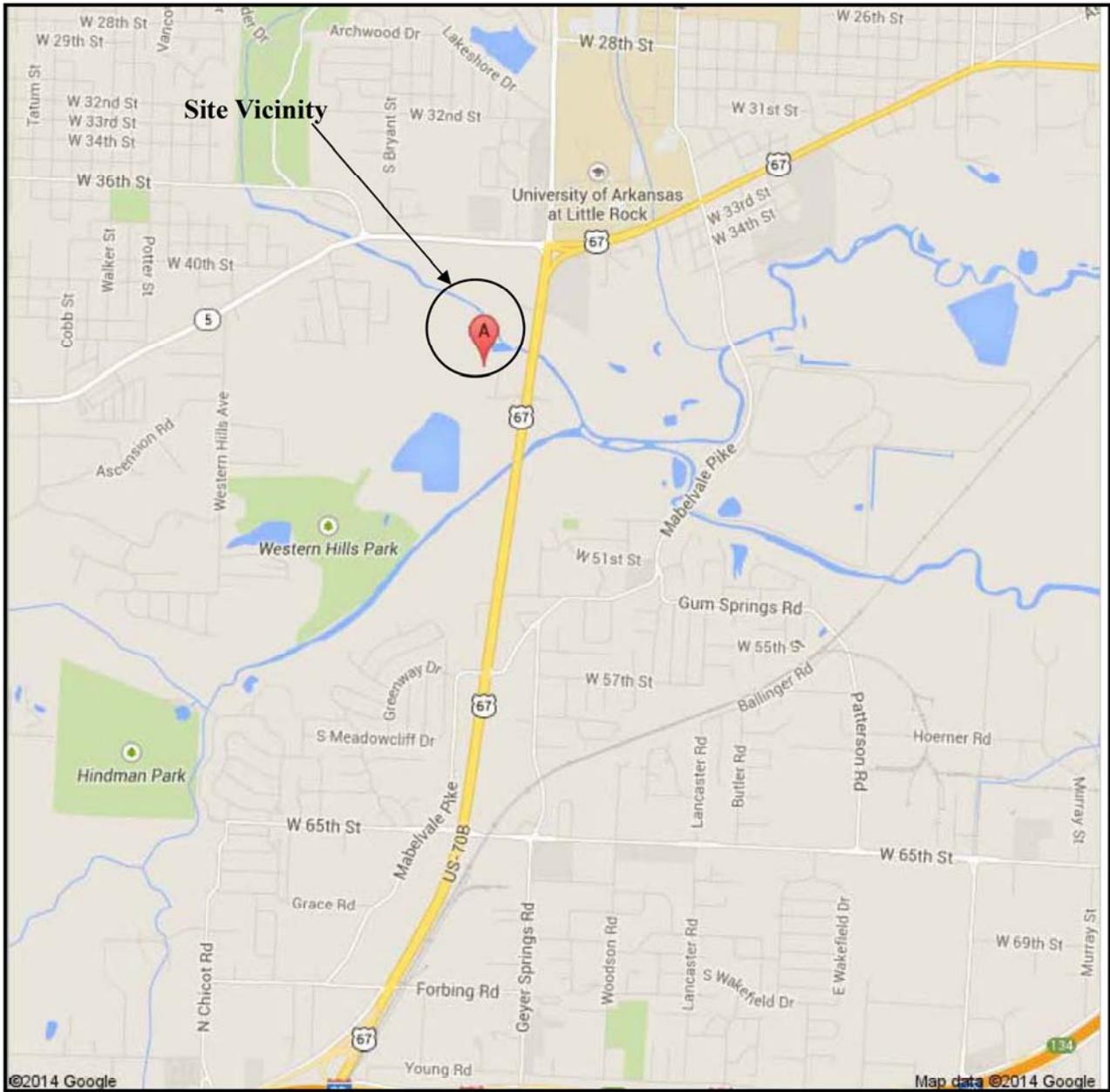


Mark E. Wyatt, P.E.
President



MEW:jw

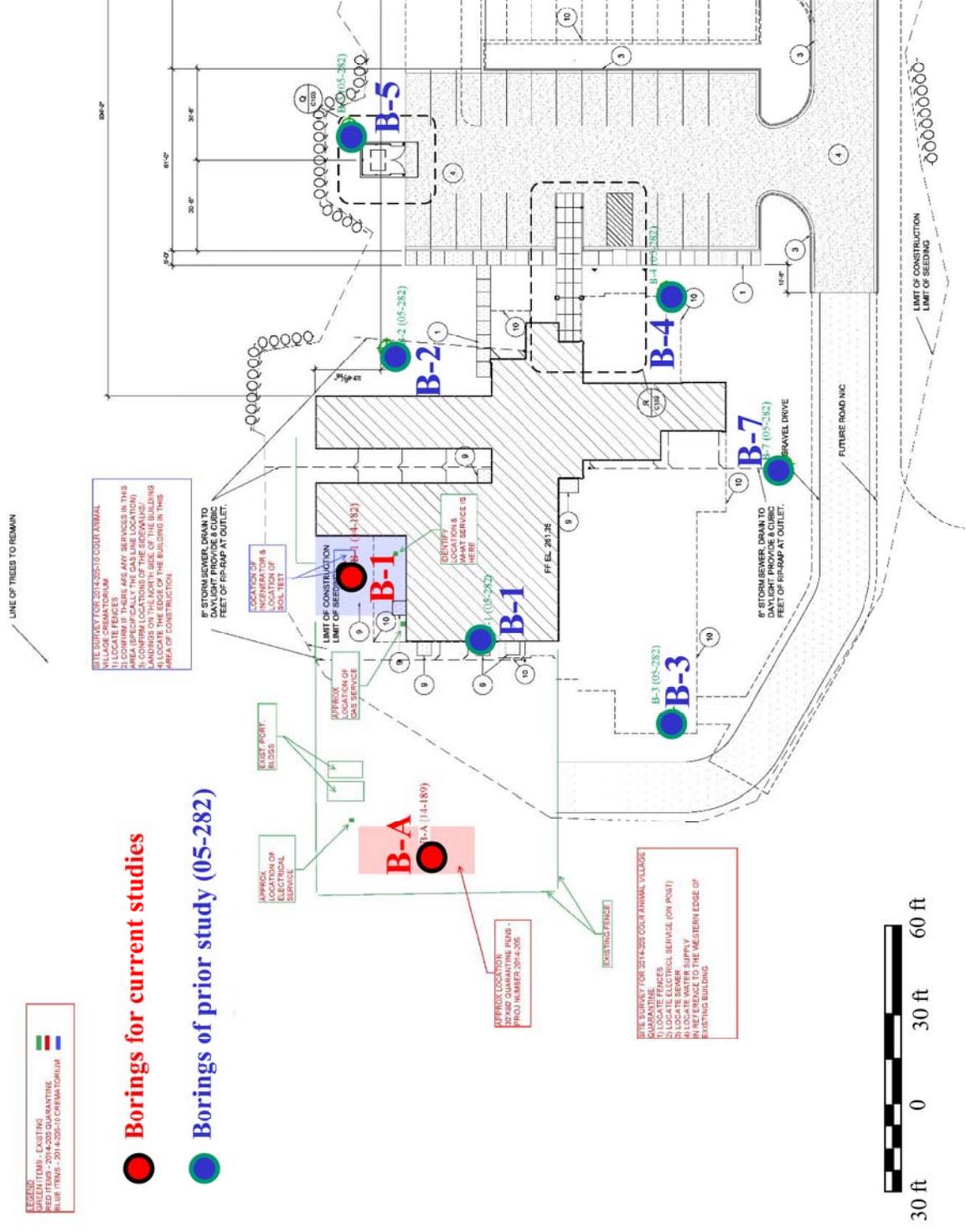
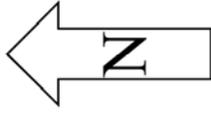
Copies submitted: Cromwell Architects Engineers
Attn: Mr. Mark Littrell, AIA (1+email)
Attn: Mr. Oscar Barrios, P.E. (1-email)



SITE VICINITY MAP
Animal Village Crematorium
Little Rock, Arkansas

Job No. 14-182

Plate 1



LEGEND
 GREEN ITEMS - EXISTING
 RED ITEMS - 2014-2025 QUARANTINE
 BLUE ITEMS - 2014-2025 CORE AREA TORNADO

- Borings for current studies
- Borings of prior study (05-282)

SITE SURVEY FOR 2014-2025 CORE AREA TORNADO VILLAGE CREMATORIUM
 1) LOCATE UTILITIES
 2) CONFIRM LOCATIONS OF THE SEWER/STORM SEWER AND GAS LINES
 3) CONFIRM LOCATIONS OF THE GAS LINE LOCATION
 4) LOCATE THE EDGE OF THE BUILDING IN THIS AREA OF CONSTRUCTION

IF STORM SEWER DRAIN TO DAYLIGHT PROVIDE 6" CURB FEET OF RIP-RAP AT OUTLET.
 LOCATION OF INCINERATOR & SCALING UNIT OF SCAL 1811

APPROX. LOCATION OF ELECTRICAL SERVICE
EXISTING PORT. CROSS

APPROX. LOCATION OF GAS SERVICE
EXISTING PORT. CROSS

IF STORM SEWER DRAIN TO DAYLIGHT PROVIDE 6" CURB FEET OF RIP-RAP AT OUTLET.
 LOCATION OF INCINERATOR & SCALING UNIT OF SCAL 1811

EXISTING PORT. CROSS
 2014-2025 QUARANTINE PINS - PROJ. NUMBER 2014-2025

IF STORM SEWER DRAIN TO DAYLIGHT PROVIDE 6" CURB FEET OF RIP-RAP AT OUTLET.
 LOCATION OF INCINERATOR & SCALING UNIT OF SCAL 1811

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IF STORM SEWER DRAIN TO DAYLIGHT PROVIDE 6" CURB FEET OF RIP-RAP AT OUTLET.
 LOCATION OF INCINERATOR & SCALING UNIT OF SCAL 1811



Scale: As Shown
Job No.: 14-182
Plate 2

PLAN OF BORINGS
Animal Village Crematorium
Little Rock, Arkansas

**Grubbs, Hoskyn,
 Barton & Wyatt, INC.**
 CONSULTING ENGINEERS



**Grubbs, Hoskyn,
Barton & Wyatt, Inc.**
Consulting Engineers

LOG OF BORING NO. 1

Animal Village Crematorium
Little Rock, Arkansas

TYPE: Auger

LOCATION: See Plate 2

DEPTH, FT	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	BLOWS PER FT	UNIT DRY WT LB/CU FT	COHESION, TON/SQ FT			- No. 200 %				
						PLASTIC LIMIT	WATER CONTENT	LIQUID LIMIT					
SURF. EL: 266±						0.2	0.4	0.6	0.8	1.0	1.2	1.4	
						+			+				
						10	20	30	40	50	60	70	
			Loose gray crushed syenite fines (fill) - with silty clay pockets at 1 ft	5					●				
				6					●				
5			- medium dense below 4 ft	14					●				
			Stiff tan silty clay, slightly sandy w/rootlets and decayed organics (fill)	15					●	+			81
			Medium dense brown, tan and gray clayey fine to coarse sand w/shale and quartz fragments (fill)	24					●	+	+		22
10			Firm brown fine sandy clay										
			- with decayed wood at 14 ft	9					●				
15			Firm tan and light gray fine sandy clay w/trace fine gravel										
			- auger refusal at 17.5 ft						●				
20													
25													

COMPLETION DEPTH: 17.5 ft
DATE: 12-4-14

DEPTH TO WATER
IN BORING: Dry

DATE: 12/4/2014

LGBNEW 14-182.GPJ 12-24-14



SYMBOLS AND TERMS USED ON BORING LOGS

SOIL TYPES

(SHOWN IN SYMBOLS COLUMN)



Gravel



Sand



Silt

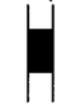


Clay

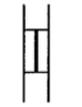
Predominant type shown heavy

SAMPLER TYPES

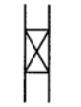
(SHOWN ON SAMPLES COLUMN)



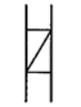
Shelby
Tube



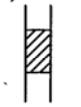
Rock
Core



Split
Spoon



No
Recovery



Cutting

TERMS DESCRIBING CONSISTENCY OR CONDITION

COARSE GRAINED SOILS (major portion retained on No. 200 sieve): Includes (1) Clean gravels and sands, and (2) silty or clayey gravels and sands. Condition is rated according to relative density, as determined by laboratory tests.

DESCRIPTIVE TERM	N-VALUE	RELATIVE DENSITY
VERY LOOSE	0-4	0-15%
LOOSE	4-10	15-35%
MEDIUM DENSE	10-30	35-65%
DENSE	30-50	65-85%
VERY DENSE	50 and above	85-100%

FINE GRAINED SOILS (major portion passing No. 200 sieve): Includes (1) Inorganic and organic silts and clays, (2) gravelly, sandy, or silty clays, and (3) clayey silts. Consistency is rated according to shearing strength, as indicated by penetrometer readings or by unconfined compression tests.

DESCRIPTIVE TERM	UNCONFINED COMPRESSIVE STRENGTH TON/SQ. FT.
VERY SOFT	Less than 0.25
SOFT	0.25-0.50
FIRM	0.50-1.00
STIFF	1.00-2.00
VERY STIFF	2.00-4.00
HARD	4.00 and higher

NOTE: Slickensided and fissured clays may have lower unconfined compressive strengths than shown above, because of planes of weakness or cracks in the soil. The consistency ratings of such soils are based on penetrometer readings.

TERMS CHARACTERIZING SOIL STRUCTURE

SLICKENSIDED - having inclined planes of weakness that are slick and glossy in appearance.

FISSURED - containing shrinkage cracks, frequently filled with fine sand or silt; usually more or less vertical.

LAMINATED - composed of thin layers of varying color and texture.

INTERBEDDED - composed of alternate layers of different soil types.

CALCAREOUS - containing appreciable quantities of calcium carbonate.

WELL GRADED - having a wide range in grain sizes and substantial amounts of all intermediate particle sizes.

POORLY GRADED - predominantly of one grain size, or having a range of sizes with some intermediate sizes missing.

Terms used on this report for describing soils according to their texture or grain size distribution are in accordance with the UNIFIED SOIL CLASSIFICATION SYSTEM, as described in Technical Memorandum No.3-357, Waterways Experiment Station, March 1953

APPENDIX A



**Grubbs, Hoskyn,
Barton & Wyatt, Inc.**
Consulting Engineers

LOG OF BORING NO. A

Animal Village Quarantine Pens
Little Rock, Arkansas

TYPE: Auger

LOCATION: See Plate 2

DEPTH, FT	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	BLOWS PER FT	UNIT DRY WT LB/CU FT	COHESION, TON/SQ FT			- No. 200 %
						PLASTIC LIMIT	WATER CONTENT	LIQUID LIMIT	
			SURF. EL: 262±						
			Stiff brown, tan and gray fine sandy clay w/trace fine gravel (fill)	20					
			- very stiff, tan and light gray below 2 ft	28					73
5			Stiff gray silty clay w/trace fine gravel (fill)	18					86
			- with wood debris at 5 ft						
			Very stiff brown silty clay w/trace decayed organics	34					
10			Stiff tan and light gray fine sandy clay, silty w/trace fine gravel	14					62
15			Dense to very dense tan clayey fine gravel, sandy	50/2"					
			- auger refusal at 15 ft						
			NOTE: Water at 6 ft at completion.						
20									
25									

COMPLETION DEPTH: 15.0 ft
DATE: 12-4-14

DEPTH TO WATER
IN BORING: 6 ft

DATE: 12/4/2014

LGBNEW 14-189.GPJ 12-24-14



**Grubbs, Hoskyn,
Barton & Wyatt, Inc.**
Consulting Engineers

LOG OF BORING NO. 1

Animal Shelter
Little Rock, Arkansas

TYPE: Auger

LOCATION: See Plate 2

DEPTH, FT	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	BLOWS PER FT	UNIT DRY WT LB/CU FT	COHESION, TON/SQ FT			- No. 200 %				
						PLASTIC LIMIT	WATER CONTENT	LIQUID LIMIT					
			SURF. EL:			0.2	0.4	0.6	0.8	1.0	1.2	1.4	
						+	+	+	+	+	+	+	
5			Very stiff tan and gray silty clay (CL) w/fine to coarse gravel and shale fragments (fill)	37									65
				31									
			Medium dense tan silt (ML) w/gray silty clay pockets	27									
10			Very stiff tan and gray silty clay (CL) w/decayed organics and ferrous nodules and stains - water at 8 ft - firm with some sandstone fragments below 8 ft	14									
				9									
15			Dense tan and gray clayey medium sand (SC) and fine to coarse gravel - with sandstone cobbles below 14.5 ft	50/5"									
			Soft dark gray slightly weathered shale	50/3"									
20													
25													

COMPLETION DEPTH: 19.0 ft
DATE: 7-14-05

DEPTH TO WATER
IN BORING: 8 ft

DATE: 7/14/2005

LGBNEW_05-282.GPJ_8-15-05



**Grubbs, Hoskyn,
Barton & Wyatt, Inc.**
Consulting Engineers

LOG OF BORING NO. 2

Animal Shelter
Little Rock, Arkansas

TYPE: Auger

LOCATION: See Plate 2

DEPTH, FT	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	BLOWS PER FT	UNIT DRY WT LB/CU FT	COHESION, TON/SQ FT				- No. 200 %			
						0.2	0.4	0.6	0.8		1.0	1.2	1.4
			SURF. EL:			+	+	+	+				
						10	20	30	40	50	60	70	
			Dense tan and gray clayey fine sand (SC) w/shale fragments and fine to coarse gravel (fill)	38		●							
				37		●	+	+					38
5			Stiff gray silty clay (CL) w/decayed organics, ferrous stains, and tan silt pockets	17			●						
			- with ferrous nodules below 7 ft	14				●					
10			Firm to stiff gray fine sandy clay (CL) w/fine to coarse sandstone gravel	10				●					
			Dense tan and gray clayey fine sand (SC) and fine to coarse gravel w/sandstone cobbles										
15			- auger refusal at 14 ft	30/0"									
20													
25													

LGBNEW_05-282.GPJ_8-15-05

COMPLETION DEPTH: 14.0 ft
DATE: 7-14-05

DEPTH TO WATER
IN BORING: Dry

DATE: 7/14/2005



**Grubbs, Hoskyn,
Barton & Wyatt, Inc.**
Consulting Engineers

LOG OF BORING NO. 3

Animal Shelter
Little Rock, Arkansas

TYPE: Auger

LOCATION: See Plate 2

DEPTH, FT	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	BLOWS PER FT	UNIT DRY WT LB/CU FT	COHESION, TON/SQ FT			- No. 200 %				
						PLASTIC LIMIT	WATER CONTENT	LIQUID LIMIT					
			SURF. EL:			0.2	0.4	0.6	0.8	1.0	1.2	1.4	
						+	+	+	+	+	+	+	
						10	20	30	40	50	60	70	
5			Medium dense tan fine sand (SP) w/sandstone cobbles and shale fragments (fill) - dense below 2 ft	18 50/4" 50/6"									
			Medium dense gray silt (ML) w/decayed organics Stiff tan and gray silty clay (CL) w/decayed organics, fine sand and ferrous stains	16									86
10			- water at 9 ft Firm tan and gray fine sandy clay (CL) w/ferrous stains and fine to coarse sandstone gravel	7									
15			Dense tan and gray clayey fine sand (SC) and fine to coarse gravel w/sandstone cobbles	50/5"									
			- auger refusal at 16 ft										
20													
25													

COMPLETION DEPTH: 16.0 ft
DATE: 7-14-05

DEPTH TO WATER
IN BORING: 9 ft

DATE: 7/14/2005

LGBNEW_05-282.GPJ_8-15-05



**Grubbs, Hoskyn,
Barton & Wyatt, Inc.**
Consulting Engineers

LOG OF BORING NO. 4

Animal Shelter
Little Rock, Arkansas

TYPE: Auger

LOCATION: See Plate 2

DEPTH, FT	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	BLOWS PER FT	UNIT DRY WT LB/CU FT	COHESION, TON/SQ FT			- No. 200 %				
						PLASTIC LIMIT	WATER CONTENT	LIQUID LIMIT					
			SURF. EL:			0.2	0.4	0.6	0.8	1.0	1.2	1.4	
						+	+	+	+	+	+	+	
						10	20	30	40	50	60	70	
			Very stiff tan and gray silty clay (CL) w/shale fragments, fine to coarse gravel, and ferrous stains (fill)	42									
				50/10"									
5			Stiff tan and gray silty clay (CL) w/ferrous and organic stains and occasional fine gravel	20									89
			- silt seam at 6.5 ft	10									
10				12									
			Soft tan and gray fine sandy clay (CL) - water at 11 ft										
			Dense tan and gray clayey fine sand (SC) and fine to coarse gravel	50/5"									
15			- with sandstone cobbles below 15 ft										
			Soft dark gray slightly weathered shale	50/3"									
20													
25													

COMPLETION DEPTH: 19.0 ft
DATE: 7-14-05

DEPTH TO WATER
IN BORING: 11 ft

DATE: 7/14/2005

LGBNEW_05-282.GPJ_8-15-05



**Grubbs, Hoskyn,
Barton & Wyatt, Inc.**
Consulting Engineers

LOG OF BORING NO. 5

Animal Shelter
Little Rock, Arkansas

TYPE: Auger

LOCATION: See Plate 2

DEPTH, FT	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	BLOWS PER FT	UNIT DRY WT LB/CU FT	COHESION, TON/SQ FT					- No. 200 %	
						0.2	0.4	0.6	0.8	1.0		1.2
SURF. EL:												
			Very stiff tan and gray fine sandy clay (CL) w/fine to coarse gravel and sandstone cobbles (fill)	50/10"								57
			- with shale fragments below 2.5 ft	26								
5			Loose gray silt (ML) w/decayed organics									
			Firm brown silty clay (CL) w/decayed organics and ferrous stains	10								
10												
15												

LGBNEW_05-282.GPJ_8-15-05

COMPLETION DEPTH: 5.5 ft
DATE: 7-14-05

DEPTH TO WATER
IN BORING: Dry

DATE: 7/14/2005

SECTION 00 41 00

BID PROPOSAL FORM

FROM: _____

(hereinafter called "Bidder")

TO: City of Little Rock Purchasing
Attention: Abdoul Kabaou
500 W Markham St, Suite 301, Little Rock, AR 72201

(hereinafter called "Owner")

RE: CREMATORIUM ADDITION- CoLR Bid# 15128
For Animal Village, City of Little Rock, Arkansas
Architect Engineer Project No. 2014-205-10

The Undersigned, having received and examined the Drawings and the Specifications for the above-referenced Project proposes to furnish all labor, materials, equipment, supervision and all associated and related items required for the Work, as required by and in strict accordance with the above-named documents for the following sum:

1.1 BASE PROPOSAL

Bidder agrees to perform all of the Work necessary to perform all of the Work necessary to complete the Total Project as indicated on the Drawings and as described in the Project Manual for the sum of:

_____ \$ _____

(Amount shall be indicated in both words and figures. In case of discrepancy, the amount indicated in words shall govern.)

1.2 ADDENDA

The Undersigned has received and examined the following Addenda numbered _____, and has incorporated the provisions in this Bid.

(If no Addenda have been issued, write in "NONE")

1.3 FURTHER CONDITIONS

The Undersigned, by submitting this Bid, further agrees:

- A. That this Bid shall be valid and may not be withdrawn within 30 days after the scheduled closing time for receiving bids; however, the prices may remain firm for a longer period of time if mutually agreeable between Bidder and the City Purchasing Division.
- B. To accept the provisions of the "NOTICE TO BIDDERS" and the "INSTRUCTIONS TO BIDDERS."
- C. To enter into and execute a Contract, if awarded on the basis of this Proposal.
- D. To furnish the Performance Bond and Labor and Material Payment Bond in accordance with the General Conditions of the Contract.
- E. To accomplish the Work in accordance with the Contract Documents, of which this Proposal is made a part.

- F. That Bidder has included all costs for complying with requirements for paying prevailing wages as determined by Arkansas Department of Labor and for compliance with Arkansas Code Annotated Section 22-9-301 et seq.
- G. Upon receipt of a Notice to Proceed to perform the designated Work expeditiously and with adequate forces to completion within 300 calendar days, including anticipated weather delay days.
- H. That the Owner will suffer financial damage if the Project is not Substantially Complete on the date mutually agreed upon as set forth in the Contract Documents. Liquidated Damages shall be assessed beginning on the first day following the maximum delivery or completion time entered on the bid form and/or provided for by the plans and specifications. The Undersigned shall pay to the Owner the sum indicted below as fixed, agreed, and liquidated damages:

\$ 250 per calendar day for each day of delay until the Work is Substantially Complete.

- I. That the City of Little Rock selects its board and commission members through a process that utilizes an executive session. That under Arkansas law, this fact deems a volunteer an employee for a limited purpose. That the City cannot contract with an employee, and cannot contract with a corporation with an employee in an executive or managerial position who also serves as a volunteer on a City board or commission unless it first passes an ordinance to approve the contract. Is any person involved with this Bid is an employee of the City, or a volunteer board or commission member who also holds an executive or managerial position with the Bidder? If the answer is "yes," please attach to this bid a disclosure statement on Bidder's letterhead identifying the person(s) and the nature of the relationship. BIDDER UNDERSTANDS THAT THIS DOES NOT MEAN that the Bidder is disqualified; but, that the apparent successful Bidder will not be selected if the Board of Directors fails to pass an ordinance to authorize the contract, regardless of the amount.
- J. To coordinate and schedule all Work with Owner.
- K. That it is understood that the Owner may reject any or all bids and waive any informalities or irregularities.

1.4 SUBCONTRACTORS

In accordance with Arkansas Statutes, §§ 22-9-204, et seq, of the State of Arkansas, I (We) submit the names of the following subcontractors, we propose to use, and their State Contractor License Nos.

	<u>Name</u>	<u>No.</u>
A.	MECHANICAL WORK - HVAC (indicative of heating, air conditioning, and ventilating)	
	_____	_____
B.	PLUMBING WORK	
	_____	_____
C.	ELECTRICAL WORK (indicative of wiring and illuminating fixtures)	
	_____	_____
D.	ROOFING & SHEET METAL (indicative of roofing application)	
	_____	_____

1.5 ATTACHMENTS

Enclosed with this bid proposal are the following:

- Conflict of Interest/Disclosure Statement *(if required, submit on Bidder's letterhead)*
- Bid Security
- Power of Attorney for Bid Bond for the Bid Security

Respectfully submitted:

Name of Firm: _____

By: _____ Date: _____

Title: _____ Contractor License No. _____

Business Address: _____

Telephone Number: (____) _____

END OF SECTION

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SECTION 32 31 13
CHAIN LINK FENCES

PART 1 GENERAL

1.01 SECTION INCLUDES

- A. Fence framework, fabric, and accessories.
- B. Fencing to match fencing for the Animal Quarantine Pens.

1.02 REFERENCE STANDARDS

- A. ASTM A123/A123M - Standard Specification for Zinc (Hot-Dip Galvanized) Coatings on Iron and Steel Products; 2013.
- B. ASTM A153/A153M - Standard Specification for Zinc Coating (Hot-Dip) on Iron and Steel Hardware; 2009.
- C. ASTM F567 - Standard Practice for Installation of Chain-Link Fence; 2011.
- D. ASTM F668 - Standard Specification for Polyvinyl Chloride (PVC) and Other Organic Polymer-Coated Steel Chain-Link Fence Fabric; 2011.
- E. ASTM F1083 - Standard Specification for Pipe, Steel, Hot-Dipped Zinc-Coated (Galvanized) Welded, for Fence Structures; 2010.

1.03 SUBMITTALS

- A. Product Data: Provide data on fabric, posts, accessories, fittings and hardware.
- B. Shop Drawings: Indicate plan layout, spacing of components, post foundation dimensions, hardware anchorage, and schedule of components.
- C. Manufacturer's Installation Instructions: Indicate installation requirements.

PART 2 PRODUCTS

2.01 MATERIALS

- A. Materials and Components: Conform to CLFMI Product Manual, www.chainlinkinfo.org.
- B. Posts, Rails, and Frames: Polymer-coated, color to match fence fabric, ASTM F1083 Schedule 40 hot-dipped galvanized steel pipe, welded construction, minimum yield strength of 30 ksi.
- C. Wire Fabric: ASTM F668 polymer-coated steel chain link fabric.

2.02 COMPONENTS

- A. Line Posts: 2.38 inch diameter.
- B. Corner Posts: 2.38 inch diameter.
- C. Fabric: 2 inch diamond mesh interwoven wire, 11 gage, 0.0907 inch thick, top selvage knuckle end closed, bottom selvage twisted tight.
- D. Tension Wire: 6 gage, 0.1620 inch thick steel, single strand.
- E. Tie Wire: Aluminum alloy steel wire.

2.03 ACCESSORIES

- A. Fittings: Sleeves, bands, clips, rail ends, tension bars, fasteners and fittings; steel.
- B. Privacy Slats: Vinyl strips, sized to fit fabric weave. Locate where indicated. Color to match fence color.

2.04 FINISHES

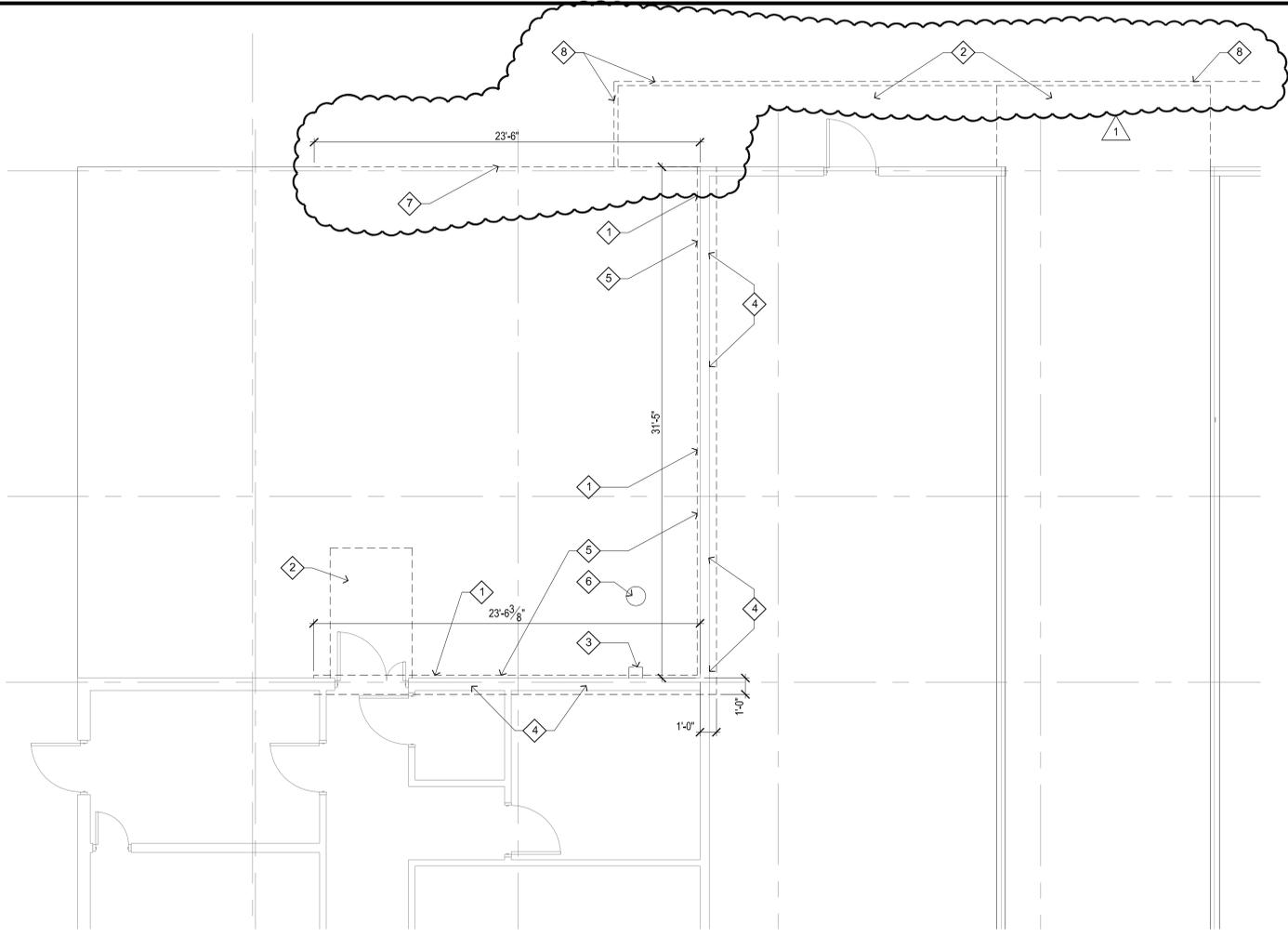
- A. Components (Other than Fabric): Galvanized in accordance with ASTM A123/A123M, at 1.7 oz/sq ft.
- B. Fabric: Vinyl coated over coating of 1.8 oz/sq ft galvanizing.
- C. Hardware: Hot-dip galvanized to weight required by ASTM A153/A153M.
- D. Color(s): To be selected by Architect Engineer from manufacturer's standard range.

PART 3 EXECUTION

3.01 INSTALLATION

- A. Install framework, fabric, accessories in accordance with ASTM F567.
- B. Place fabric on centerline of posts and rails.
- C. Install tie wires, tension wires, clips, and other accessories as necessary to prevent injury to animals and people.
- D. Do not stretch fabric until concrete foundation has cured 28 days.
- E. Position bottom of fabric 1 inches above finished masonry or slab.
- F. Fasten fabric to bottom rail, line posts, braces, and top tension wire with tie wire at maximum 15 inches on centers.
- G. Attach fabric to end, corner, and gate posts with tension bars and tension bar clips.
- H. Install top tension wire stretched taut between posts.

END OF SECTION



- KEYED DEMOLITION NOTES:**
- 1 REMOVE EXISTING VINYL SIDING TO THE DIMENSIONS SHOWN. PREPARE AREA FOR NEW CONSTRUCTION.
 - 2 REMOVE EXISTING CONCRETE SIDE WALK TO THE DIMENSIONS SHOWN. PREPARE AREA FOR NEW CONSTRUCTION.
 - 3 REMOVE EXISTING WALL MOUNTED LIGHT FIXTURE (SEE ELECT). PREPARE AREA FOR NEW CONSTRUCTION.
 - 4 REMOVE EXISTING ROOFING BACK TO DIMENSION SHOWN. PROVIDE TEMPORARY PROTECTION DURING CONSTRUCTION. PREPARE AREA FOR NEW CONSTRUCTION.
 - 5 REMOVE EXISTING ROOF FLASHING TO THE DIMENSIONS SHOWN. PREPARE AREA FOR NEW CONSTRUCTION.
 - 6 RELOCATE EXISTING CLEANOUT (SEE PLUMB.)
 - 7 REMOVE EXISTING FENCING TO THE DIMENSION SHOWN.
 - 8 REMOVE EXISTING VINYL COATED FENCING (AS REQUIRED FOR WORK ON THE SIDEWALK REPAIR). REPLACE TO MATCH ORIGINAL.

- GENERAL DEMOLITION NOTES:**
- 1. PATCH, REPAIR AND/OR REPLACE ANY AREAS AFFECTED BY DEMOLITION
 - 2. SAW CUT MATERIALS TO REMAIN IN PLACE AT EXTENT OF DEMOLITION
 - 3. PROTECT ALL AREAS FROM DAMAGE DURING DEMOLITION

1 DEMOLITION FLOOR PLAN
AD101

No.	Date	Description
1	04-13-2015	ABERDUM001



CONSTRUCTION DOCUMENTS

CITY OF LITTLE ROCK
ANIMAL VILLAGE
CREMATORIUM ADDITION



101 south spring street little rock, ar 72201
501.372.2900 cromwell.com

Issue Date: 30-JANUARY-2015 Project Number: 2014-205-10

DEMOLITION FLOOR PLAN

AD101